



# Calculating potential of water harvesting from road catchments

# Introduction

- Water harvesting is the collection of runoff for a productive use (Critchley and Siegert, 1999)
- Unless it is properly managed, **concentrated** water from road catchments can be damaging
- Some of the negative effects of concentrated water emanating from culverts and bridges are:
  - ✓ Downstream erosion of land leading to gully formation
  - ✓ Downstream deposition of sediments
  - ✓ Damaging road structures, mainly hydraulic structures

# Introduction



Fig. Example of gully formation downstream of a road along the Freweyni to Abreha-we-Atsibha road route

# Introduction



# Introduction

- With proper planning and management, concentrated water from road catchments could be utilized for different agricultural uses:
  - ✓ Diverted to farmlands for supplemental irrigation
  - ✓ Channelled to deep trenches, percolation ponds, etc. for ground water recharge
- With known water **potential to harvest from road catchments**, it would be possible to manage the available water for **different water uses** and to **size different water harvesting systems**.

# Calculating potential of water harvesting form road catchments

- The Runoff Volume Equation for calculating the **potential of water harvesting (m<sup>3</sup>)** is

$$V = C * P * A * E$$

Where,  $V$  = is potential of water harvesting (m<sup>3</sup>)

$C$  = weighted runoff coefficient for the catchment area

$P$  = Dependable annual rainfall, usually 80% (m)

$E$  = Efficiency factor for accounting losses, usually taken 0.7 (Danida, 2006)

- Water losses could be due to evaporation and seepage

# Estimation of annual dependable rainfall

- Requires records of annual precipitation from meteorological station/s nearby to the road
- Determine the most probable annual rainfall for the area
- Nowadays 80% of dependable rainfall is taken
- Annual rainfall records arranged in descending order, and ranked from 1 to n, in which n belongs to the number of annual rainfall records
- Probability of exceedance computed using probability distributions, for example, Weibul formula
- $P = m / (n + 1)$

where m is the rank of each rainfall for a given year based on ranking on descending order

# Calculating potential of water harvesting from road catchments

- Example: Annual rainfall records for the Freweyni Station, Tigray

| Year | Rainfall (mm) |
|------|---------------|
| 2001 | 778.0         |
| 2002 | 405.2         |
| 2003 | 466.9         |
| 2004 | 604.6         |
| 2005 | 341.9         |
| 2006 | 642.0         |
| 2007 | 537.0         |
| 2008 | 567.6         |
| 2009 | 306.7         |
| 2010 | 670.5         |
| 2011 | 623.6         |
| 2012 | 549.0         |

| Year | Rainfall (mm) | Rank     | Probability of exceedance = $m/(n+m)$ |
|------|---------------|----------|---------------------------------------|
| 2001 | 778.0         | 1        | 0.08                                  |
| 2010 | 670.5         | 2        | 0.15                                  |
| 2006 | 642.0         | 3        | 0.23                                  |
| 2011 | 623.6         | 4        | 0.31                                  |
| 2004 | 604.6         | 5        | 0.38                                  |
| 2008 | 567.6         | 6        | 0.46                                  |
| 2012 | 549.0         | 7        | 0.54                                  |
| 2007 | 537.0         | 8        | 0.62                                  |
| 2003 | 466.9         | 9        | 0.69                                  |
| 2002 | 405.2         | 10       | 0.77                                  |
| 2005 | 341.9         | 11       | 0.85                                  |
| 2009 | 306.7         | 12       | 0.92                                  |
|      |               | 80% dep. | 380                                   |



# Determination of runoff coefficient

- Runoff coefficient is dimensionless that relates runoff to precipitation
- Runoff coefficient depend on land use, slope, soil type, etc.
- Runoff coefficient is determined for various **land uses** or using known **slope** and **soil hydrologic group**
- The soil hydrologic group describes the rate of water infiltration and depends on soil type

**Table 5-10: Typical Hydrologic Soils Groups for Ethiopia**

|    | Soil Types        | Hydrologic Soil Group |
|----|-------------------|-----------------------|
| Ao | Orthic Acrisols   | B                     |
| Bc | Chromic Cambisols | B                     |
| Bd | Dystric Cambisols | B                     |
| Be | Eutric Cambisols  | B                     |
| Bh | Humic Cambisols   | C                     |
| Bk | Calcic Cambisols  | B                     |
| Bv | Vertic Cambisols  | B                     |
| Ck | Calcic Chernozems | B                     |
| E  | Rendzinas         | D                     |

*Source: Drainage Design Manual (ERA, 2013)*

# Determination of runoff coefficient

**Table 5-5: Recommended Runoff Coefficient C for Pervious Surfaces by Selected Hydrologic Soil Groupings and Slope Ranges**

| Terrain Type     | Soil Type |           |           |           |
|------------------|-----------|-----------|-----------|-----------|
|                  | A         | B         | C         | D         |
| Flat, <2%        | 0.04-0.09 | 0.07-0.12 | 0.11-0.16 | 0.15-0.20 |
| Rolling, 2-6%    | 0.09-0.14 | 0.12-0.17 | 0.16-0.21 | 0.20-0.25 |
| Mountain, 6-15%  | 0.13-0.18 | 0.18-0.24 | 0.23-0.31 | 0.28-0.38 |
| Escarpment, >15% | 0.18-0.22 | 0.24-0.30 | 0.30-0.40 | 0.38-0.48 |


$$\text{Weighted } C = \frac{C1 * A1 + C2 * A2 + \dots Cn * An}{A1 + A2 + \dots An}$$

# Determination of runoff coefficient

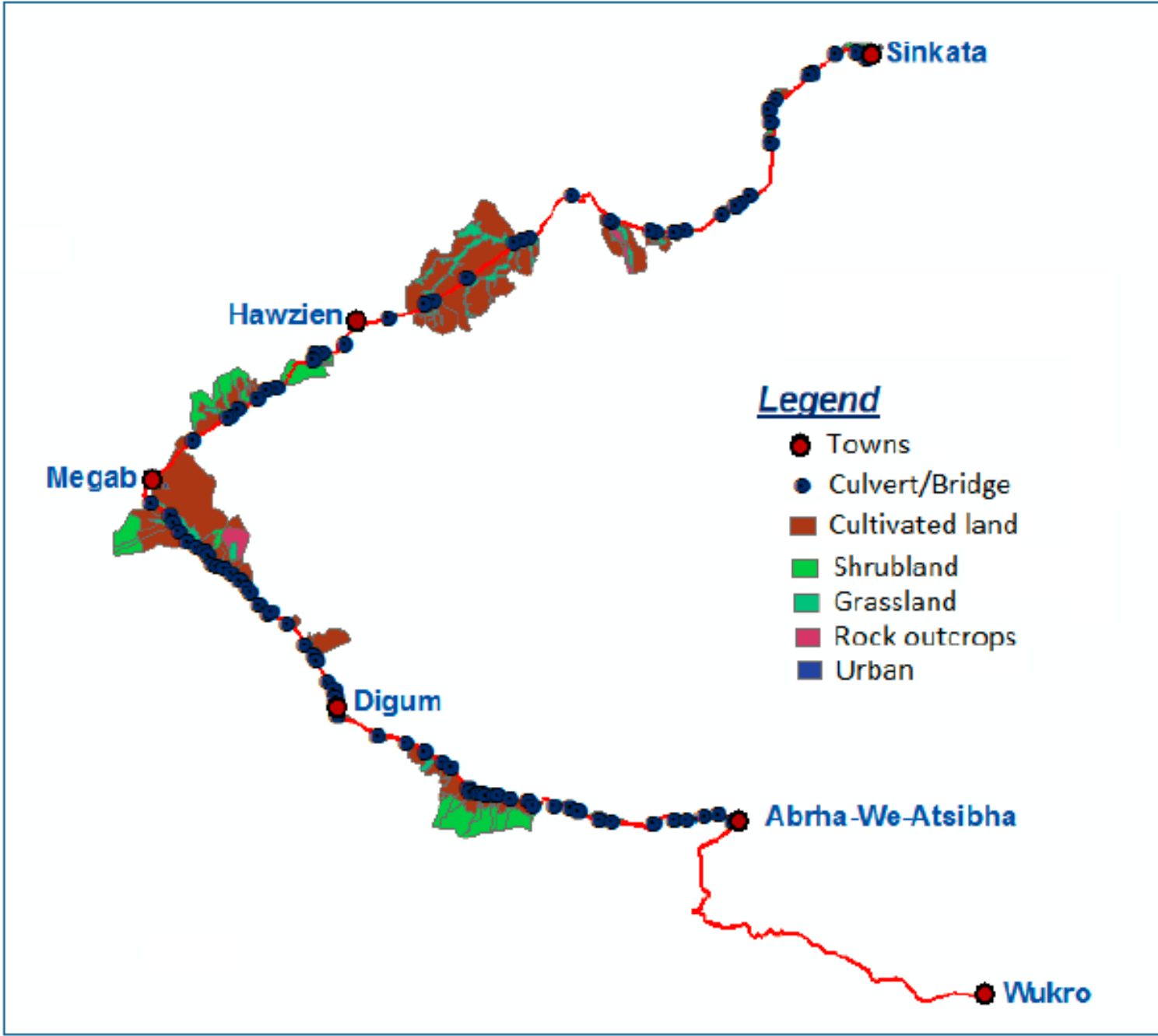
**Table 5-6: Recommended Runoff Coefficient C for Various Land Use:**

| Description of Area                    | Runoff Coefficients |
|--|---------------------|
| Business: Downtown areas               | 0.70-0.95           |
| Neighborhood areas                     | 0.50-0.70           |
| Residential: Single-family areas       | 0.30-0.50           |
| Residential: Multi units, detached     | 0.40-0.60           |
| Residential: Multi units, attached     | 0.60-0.75           |
| Suburban                               | 0.25-0.40           |
| Residential (0.5 hectare lots or more) | 0.30-0.45           |
| Apartment dwelling areas               | 0.50-0.70           |
| Industrial: Light areas                | 0.50-0.80           |
| Industrial: Heavy areas                | 0.60-0.90           |
| Parks, cemeteries                      | 0.10-0.25           |
| Playgrounds                            | 0.20-0.40           |
| Railroad yard areas                    | 0.20-0.40           |
| Unimproved areas                       | 0.10-0.30           |

(Source: Hydrology, Federal Highway Administration, HEC No. 19, 1984)



*Example: water harvesting potential from road catchments at the outlet of 92 culverts/box bridges along the **Freweyni to Abreha-we-Atsibha** route in Tigray, total potential of water harvesting is **1.34 Million cubic meter.***



# Calculating potential of water harvesting from road catchments

## ■ **General steps**

- ✓ Delineate the catchment area using, e.g. arc-hydro, ArcSWAT, using **Google Earth** (identification of water-divide based on elevation profile)
- ✓ Determine the area of each land use in the catchment
- ✓ Estimate runoff coefficient
- ✓ Estimate annual dependable rainfall
- ✓ Finally, determine the runoff volume using the **Runoff volume Equation**

# Calculating peak discharge at culvert/bridge outlets

- Peak discharge is used for **sizing the spillway** of a **water storage structure** such as ponds.
- In the absence of measured data, peak discharge is usually estimate using:
  - ✓ The **rational method** (catchment area <50 ha)
  - ✓ The SCS curve number method (catchment area >50 ha)
- Peak discharge using the rational method is estimated as:

$$Q = 0.00278 CIA$$

Where, Q: is maximum discharge (m<sup>3</sup>/s)

C: is runoff coefficient

I : is average rainfall intensity for duration equal to time of concentration, mm/hr

A: is catchment area, in ha

# Procedures for using Rational method in Ethiopia

- Determine the catchment area in hectares
- Determine the time of concentration, with consideration for future characteristics of the catchment

$$T_c = \frac{1}{3000} * \left( \frac{L}{S^{1/2}} \right)^{0.77}$$

where  $T_c$  : is time of concentration in hours

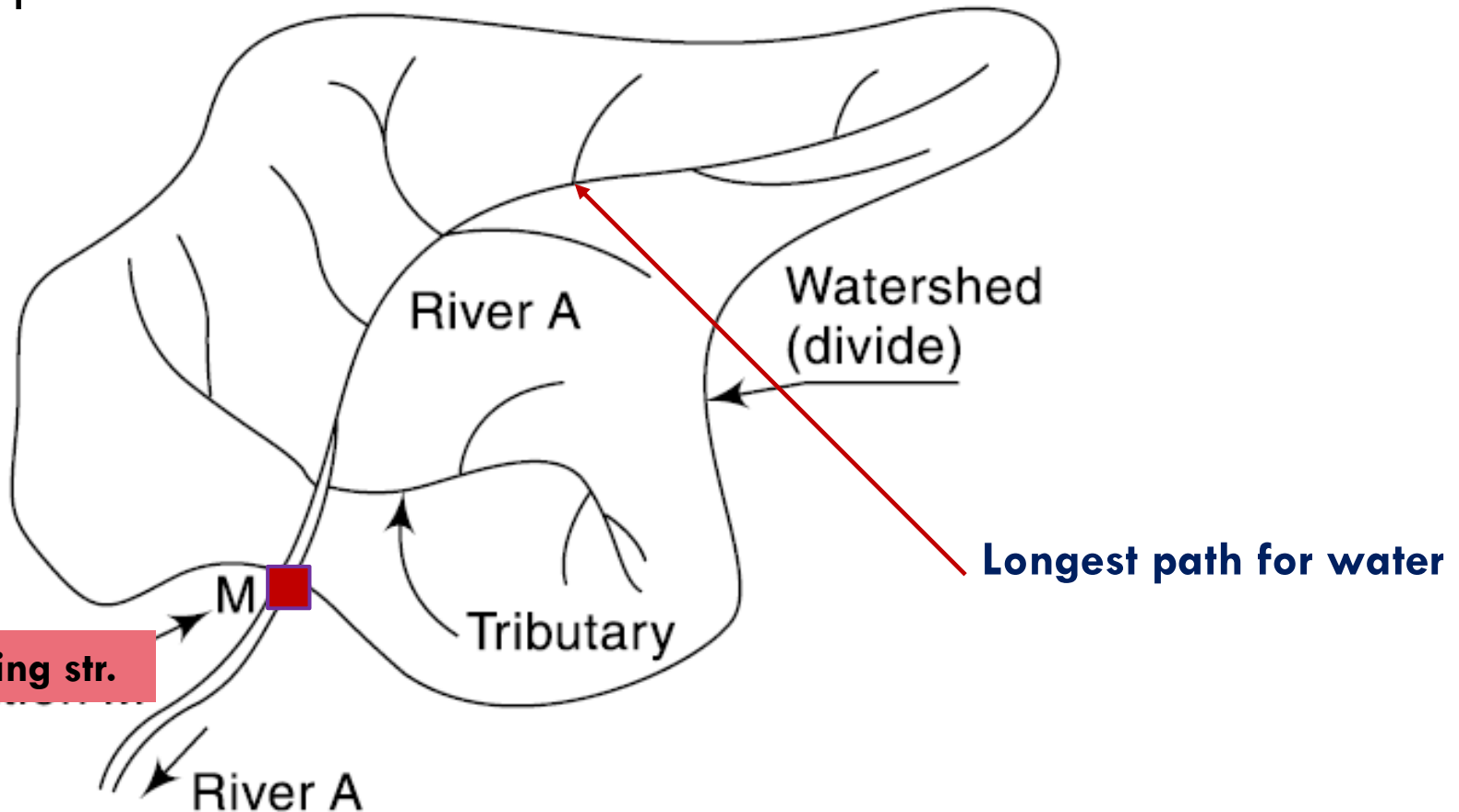
$L$  : is hydraulic length of the catchment (m),  $S$  is average slope of the catchment

- Determine rainfall intensity using the **Intensity-Duration-Frequency** (IDF) charts for a given region



# Procedures for using Rational method in Ethiopia

- Time of concentration is the time taken for water to move along the longest path in the catchment



# Procedures for using Rational method in Ethiopia

- Rainfall intensity depends on  $T_c$  and return period/design period
- Design period depends on the type of structure

| Type of structure            | Return period (years) |
|------------------------------|-----------------------|
| Highway culverts:            |                       |
| Low traffic                  | 5–10                  |
| Intermediate traffic         | 10–25                 |
| High traffic                 | 50–100                |
| Highway bridges              |                       |
| Secondary system             | 10–50                 |
| Primary system               | 50–100                |
| Urban drainage               |                       |
| Storm sewers in small cities | 2–25                  |
| Storm sewers in large cities | 25–50                 |
| Airfields                    |                       |
| Low traffic                  | 5–10                  |
| Intermediate traffic         | 10–25                 |
| High traffic                 | 50–100                |

# Procedures for using Rational method in Ethiopia

- IDF curves for different parts of Ethiopia

| Meteorological Region | Station      | Years of Record | Meteorological Region | Station       | Years of Record |    |
|-----------------------|--------------|-----------------|-----------------------|---------------|-----------------|----|
| A1                    | Axum         | 17              | B                     | Bedele        | 39              |    |
|                       | Mekele       | 46              |                       | Gore          | 56              |    |
|                       | Maychew      | 32              |                       | Nekempte      | 40              |    |
| A2                    | Gondar       | 52              |                       | Jima          | 54              |    |
|                       | Debre Tabor  | 15              |                       | Arba Minch    | 23              |    |
|                       | Bahir Dar    | 45              |                       | Sodo          | 49              |    |
|                       | Debre Markos | 55              |                       | Awasa         | 36              |    |
|                       | Fitche       | 44              |                       | C             | Kombolcha       | 57 |
|                       | Addis Ababa  | 57              |                       |               | Woldiya         | 29 |
|                       | Debre Zeit   | 55              |                       |               | Sirinka         | 27 |
| A3                    | Nazareth     | 46              | D1                    | Gode          | 33*             |    |
|                       | Kulumsa      | 43              |                       | Kebri Dihar   | 40              |    |
|                       | Robe/Bale    | 29              | D2                    | Kibre Mengist | 33              |    |
| A4                    | Metehara     | 24              |                       | Negele        | 51              |    |
|                       | Dire Dawa    | 58              |                       | Moyale        | 29              |    |
|                       | Mieso        | 42              |                       | Yabelo        | 34              |    |

# Calculating Peak discharge using Rational method

- Rainfall intensity is determined based design return period and calculated time of concentration for a catchment

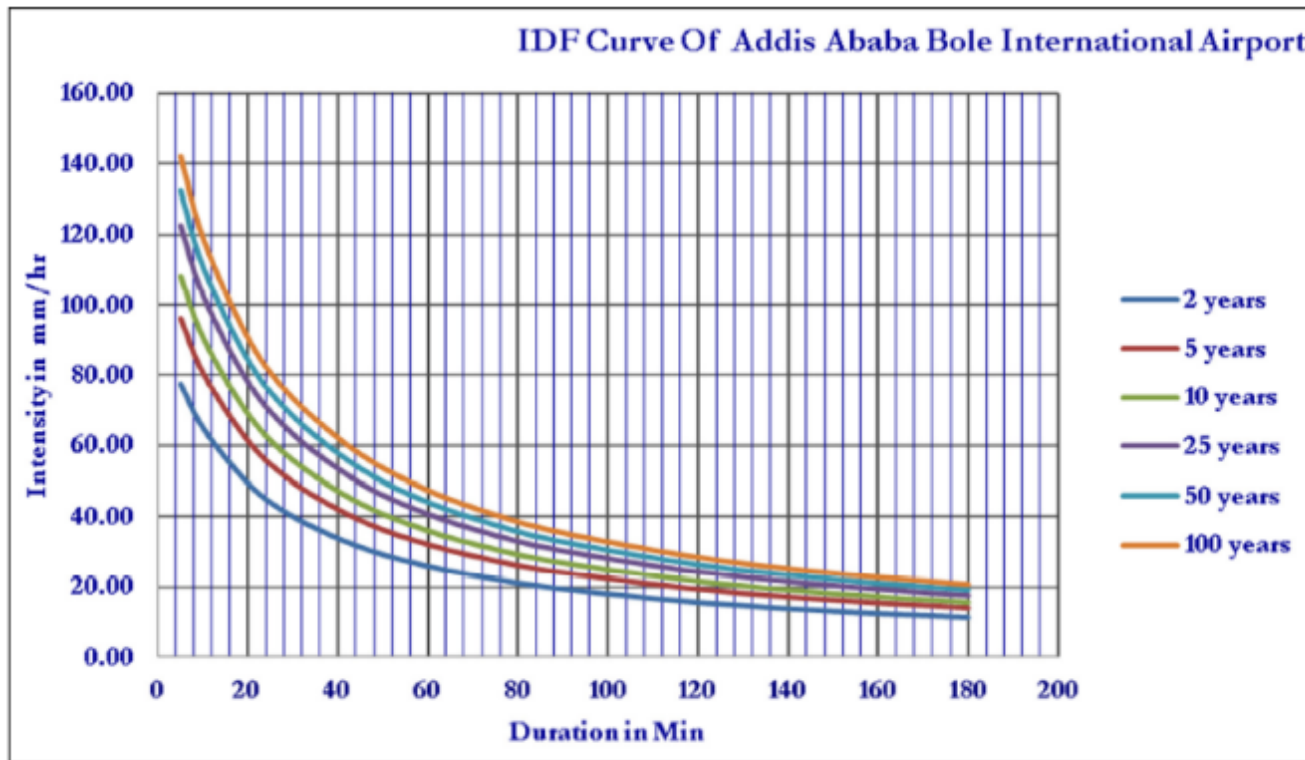


Figure 5-12: Typical Rainfall Intensity Duration Frequency Curve

# Design considerations for water conveyance/channel

- ✓ Amount of water/ peak discharge
- ✓ Terrain slope affecting speed of water
- ✓ Alignment of the channel
- ✓ Stability of channel, earthen or lined channel
- ✓ Shape of the channel (e.g. rectangular, trapezoidal, etc.)
- ✓ Minimum speed of water to avoid siltation
- ✓ Channel surface roughness
- ✓ Free board
- ✓ Size of the channel (*Design ??*)

# Design considerations for diversion channels from culverts

- *Design discharge for sizing channels*
  - ✓ *Estimate using the rational method*
  - ✓ *Calculate from existing culverts/bridges, i.e.*
  - ✓  *$Q = A.V$ , where  $A$  cross-sectional area of the culvert,  $V$  is velocity of flow*
  - ✓ *Determine velocity of flow using flow measurements techniques (e.g. current meter, floating techniques)*



Thank you!